# **Armstrong Hills**

Traffic Impact Analysis December 2024



Prepared for: Holmes Builders

Prepared by:



*Texas Board of Professional Engineers Registration No. F-23290* 6330 West Loop South, Suite 150 Bellaire, Texas 77401 (713) 777-5337

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## **Executive Summary**

Quiddity Engineering, LLC conducted a Traffic Impact Analysis (TIA) for the Armstrong Hills Development in City of Keller, Texas. Traffic impacts were analyzed for the proposed build out year, 2027, and considered AM and PM peak period traffic operations.

#### Project Background and Location

The Armstrong Hills Development is proposed on the east side of Davis Blvd (FM 1938) between Lyndhurst Way/Creek Rd and Rolling Wood Ln in City of Keller, Texas. The proposed development will include 49 single-family homes and is assumed to be completed in 2027. Access to the proposed development will be provided by one full-access roadway on Davis Blvd (FM 1938), Street A.

#### **Intersections**

The intersection included in the study area are:

- Davis Blvd (FM 1938) at Lyndhurst Way/Creek Rd
- Davis Blvd (FM 1938) at (Proposed) Street A
- Davis Blvd (FM 1938) at Rolling Wood Ln





#### Site Plan



#### Data Collection

A site visit was conducted to document the existing conditions of the study area roadways and site. Peak Hour counts (6-9 AM and 4-7 PM) were collected at existing study intersections. Peak hours were reached during the weekday between 7:30 – 8:30 AM for the AM peak hour and 4:45 – 5:45 PM for the PM peak hour.

#### Growth Rate

A 2.2% annual compound growth rate for background traffic was calculated based on available historical traffic data in the site vicinity provided by TxDOT's *Traffic Count Database System* (TCDS).

#### **Background Construction:**

The FM 1938 (Davis Boulevard) Raised Median Project is proposed by the Texas Department of Transportation (TxDOT). The project is proposing improvements to Davis Boulevard (FM 1938) from FM 1709 to Emerald Hills Way in Tarrant County, Texas. The proposed improvement will convert the twoway left-turn lane into a 14-foot-wide raised median. Per the project schematic and in coordination with TxDOT, median openings will be provided along David Boulevard (FM 1938) at Lyndhurst Way/Creek Road and at Rolling Wood Lane, but will not be provided at (proposed) Street A. Upon project completion, Street A will operate as a right-in right-out driveway. Project construction is anticipated to begin in the year 2026. The completion date for this improvement project is unknown; therefore, was not included in the analysis.



#### Trip Generation and Distribution

The Institute of Transportation Engineers (ITE), Trip Generation, 11<sup>th</sup> Edition, was used to estimate the traffic that will be generated by the proposed development by using Online Traffic Impact Study Software (OTISS). The development is estimated to generate 40 AM Peak Hour trips, 52 PM Peak Hour trips and 534 Weekday trips.

The directional distribution of the site generated trips was determined based on existing and expected travel patterns in the area. Trip origins and destinations for the proposed development are expected to be 55% to/from the north via Davis Blvd (FM 1938) and 45% to/from east via Davis Blvd (FM 1938).

#### Capacity Analysis

Capacity Analysis was performed at the study intersections for 2024 Existing Conditions, 2027 Background Conditions, and 2027 Projected Conditions using *Synchro 12*, a traffic modeling and capacity analysis software. Seconds of Delay for each approach at the study intersections were used to determine a Level of Service (LOS). All approaches are projected to operate at an acceptable LOS except for the following:

For 2027 Background Conditions, at the intersection of Davis Blvd (FM 1938) and Lyndhurst Way, the eastbound, westbound, and southbound approaches are failing with a LOS E or F for the AM Peak Hour, and the eastbound, westbound, and northbound approaches are failing with a LOS E or F for the PM Peak Hour. Although the LOS is failing, LOS E or F is maintained from 2027 Background Conditions to 2027 Projected Conditions for each approach; therefore, no mitigations are recommended.

For 2027 Background Conditions, at the intersection of Davis Blvd (FM 1938) and Rolling Wood Ln, the westbound and southbound approaches are failing with a LOS E or F for the AM Peak Hour, and the westbound approach is failing with a LOS E for the PM Peak Hour. Although the LOS is failing, LOS E or F is maintained from 2027 Background Conditions to 2027 Projected Conditions for each approach; therefore, no mitigations are recommended.

For 2027 Projected Conditions, at the intersection of Davis Blvd (FM 1938) and Street A, the westbound and southbound approaches are failing with a LOS E or F in the AM Peak Hour and the westbound approach is failing with a LOS E in the PM Peak Hour. For the westbound approach, LOS E-F on the minor driveway approach is not uncommon at an unsignalized intersection where traffic volumes on the major roadways are high, such as Davis Blvd (FM 1938). LOS E-F is for the minor roadway approaches only, not the overall intersections, and minimal volumes from the development will be traveling westbound through the intersection (1% of overall traffic at Street A in AM Peak Hour / 0.5% of overall traffic at Street A in PM Peak Hour); therefore, no mitigations are recommended. For the southbound approach, although the LOS is failing in the AM Peak Hour, minimal volumes from the development will be making southbound left turn movements (0.2% of overall traffic at Street A in AM Peak Hour). Additionally, the TxDOT FM 1938 Raised Median Project will eventually restrict the failing southbound left turn movement. Minimal volumes from the development will be making southbound left turn movement. Minimal volumes from the development will be making southbound left turn movement. Minimal volumes from the development will be making southbound left turn movement. Minimal volumes from the development will be making southbound left turn movement. Minimal volumes from the development will be making southbound left turn movements, and future background construction will restrict the southbound left turn movement; therefore, no mitigations are recommended.



#### Access Management

For 2027 Projected Conditions, right turn lane analysis was performed in accordance with Table 2-3 of the *TxDOT Access Management Manual* at the intersection of Davis Blvd (FM 1938) and Street A. The northbound approach of the intersection did not meet the threshold for a right turn lane. Although a right turn lane is required in accordance with TxDOT District Requirements, a northbound right turn lane may not be feasible due to the existing super elevation on Davis Blvd (FM 1938) and the location of (proposed) Street A is at the southern boundary of the development where any right-of-way necessary would be on an adjacent property; therefore, a northbound right turn lane is not recommended.

Table 3-1 of the American Association of State Highway and Transportation Officials' (AASHTO) Green Book provides a Stopping Sight Distance of 425 feet based on a speed limit of 50 mph and a level roadway. The northbound and southbound movements on Davis Blvd (FM 1938) are unobstructed and on a level roadway which meets the minimum 425 foot Stopping Sight Distance.

Table 9-6 and Table 9-8 of the AASHTO Green Book provide Intersection Sight Distances for Case B1, Left Turn from Stop, and Case B2, Right Turn from Stop, respectively. The minimum Intersection Sight Distance for Case B1, Left Turn from Stop, is 670 feet. The minimum Intersection Sight Distance for Case B2, Right Turn from Stop, is 480 feet. Both turning movements at the proposed roadway, Street A, are unobstructed.

#### **Driveway Spacing**

Table 2-2 of the *TxDOT Access Management Manual* provides a minimum State Highway Connection Spacing (Driveway Spacing) of 425 feet based on speed limits greater than 50 mph. On Davis Blvd (FM 1938), the distance from proposed Street A to the nearest roadway to the north, Creek Road, is 270 feet and 140 feet to the nearest roadway to the south, Rolling Wood Lane. Minimum driveway spacing requirements are not met because Street A is proposed to align between Rolling Wood Lane and Creek Road. The only available access to the proposed site is along Davis Blvd (FM 1938) between Rolling Wood Lane and Creek Road, and the driveway spacing between Rolling Wood Lane and Creek Road is approximately 525 feet, which does not allow Street A to meet driveway spacing requirements; therefore, a variance is requested for Street A.

#### **Recommendations**

Traffic impacts to the surrounding roadway are minimal and approval is recommended with the following recommendations:

1. Request a variance for driveway spacing for proposed Street A as part of the permit process.



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## Introduction

The Armstrong Hills Development is proposed on the east side of Davis Blvd (FM 1938) between Lyndhurst Way/Creek Rd and Rolling Wood Ln in City of Keller, Texas, as shown in **Figure 1**. The proposed development will include 49 single-family homes and is assumed to be completed in 2027. Access to the proposed development will be provided by one full-access roadway on Davis Blvd (FM 1938), Street A. The purpose of this study is to determine the potential traffic impacts of the proposed development on the surrounding roadways and intersections.



Figure 1 – Project Location



## **Existing Conditions**

A site visit was conducted to document the existing conditions of the study area roadways and site. Existing lane assignments and traffic control at the study area intersections can be found in Appendix A.

#### Roadways

Davis Blvd (FM 1938) is a north-south roadway and is classified as a 6 Lane Divided Arterial per the 2022 City of Keller Major Thoroughfare Plan (MTP). It is an asphalt roadway and approximately 80' in width with a 14' two-way left turn lane. It has three travel lanes in each direction and a posted speed limit of 50 mph.

Lyndhurst Way is an east-west roadway and is not classified per the 2022 City of Keller MTP. It is a concrete roadway and approximately 30' in width. It has one travel lane in each direction and a posted speed limit of 30 mph.

Creek Rd is an east-west roadway and is not classified per the 2022 City of Keller MTP. It is an asphalt roadway and approximately 20' in width. It has one travel lane in each direction and a posted speed limit of 30 mph.

Rolling Wood Ln is an east-west roadway and is not classified per the 2022 City of Keller MTP. It is a gated concrete roadway and approximately 30' in width. It has one travel lane in each direction and an assumed speed limit of 30 mph.

An excerpt of the 2022 City of Keller Major Thoroughfare Plan is provided in Figure 2.



#### Figure 2 – City of Keller Major Thoroughfare Plan

#### **Intersections**

The intersection included in the study area are:

- Davis Blvd (FM 1938) at Lyndhurst Way/Creek Rd
- Davis Blvd (FM 1938) at (Proposed) Street A
- Davis Blvd (FM 1938) at Rolling Wood Ln

#### <u>Site</u>

The existing site is vacant.

#### **Background Construction**

The FM 1938 (Davis Boulevard) Raised Median Project is proposed by the Texas Department of Transportation (TxDOT). The project is proposing improvements to Davis Boulevard (FM 1938) from FM 1709 to Emerald Hills Way in Tarrant County, Texas. The proposed improvement will convert the twoway left-turn lane into a 14-foot-wide raised median. Per the project schematic and in coordination with TxDOT, median openings will be provided along David Boulevard (FM 1938) at Lyndhurst Way/Creek Road and at Rolling Wood Lane, but will not be provided at (proposed) Street A. Upon project completion, Street A will operate as a right-in right-out driveway. Project construction is anticipated to begin in the year 2026. The completion date for this improvement project is unknown; therefore, was not included in the analysis. The FM 1938 (Davis Boulevard) Raised Median Project Fact Sheet and Schematic are provided in **Appendix G**.

#### Traffic Data

Turning movement counts were taken by CJ Hensch & Associates, Inc. on Wednesday, October 23<sup>rd</sup>, 2024. Peak hour counts were collected from 6 AM - 9 AM and 4 PM – 7 PM. Peak hours were reached during the weekday between 7:30-8:30 AM for the AM peak hour and 4:45-5:45 PM for the PM peak hour. The AM and PM Peak Hour Factor (PHF) and heavy vehicle percentage was determined for each existing intersection from collected traffic counts. The heavy vehicle percentage utilized is the percent of Articulated Trucks, Buses, and Single-Unit Trucks. A PHF of 0.92 and heavy vehicle percentage of 2% was utilized at proposed intersections.

The existing traffic volumes can be found in **Appendix A** and raw traffic count data can be found in **Appendix B**.



Growth Rate

A 2.2% average compound annual growth rate was calculated based on available traffic data in the site vicinity provided by TxDOT's Traffic Count Database System (TCDS) using the Compound Annual Growth Rate formula noted below. A summary of data provided by TCDS is provided in Table 1 and raw data is provided in Appendix B.

Compound Annual Growth Rate = 
$$\left(\frac{V_{Final}}{V_{Begin}}\right)^{\frac{1}{t}} - 1$$

Location	Year	TxDOT AADT (vpd)	Compound Annual Growth Rate
Davis Blvd	2014	29,463	1 8%
(South of Rolling Wood Ln)	2019	37,239	4.876
Davis Blvd	2009	33,860	0.4%
(North of Lyndhurst Way/Creek Rd)	2022	32,186	-0.4%
Average Gro	owth Rate:		2.2%

#### Table 1 – Growth Rate Data

#### **Trip Generation**

The Institute of Transportation Engineers (ITE), Trip Generation, 11<sup>th</sup> Edition, was used to estimate the traffic that will be generated by the proposed development by using Online Traffic Impact Study Software (OTISS). The following analysis periods were utilized:

- Weekday
- Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 AM
- Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 PM •

Table 2 provides a summary of the estimated trips generated for the proposed development and the Trip Generation Reports can be found in **Appendix F.** No trip reductions were applied.

	Table 2 – Trip Generation Volumes														
Droposod Lond Lise (ITE code)	Sizo	24-Hour		AM Peak		PM Peak									
Proposed Land Ose (TE code)	5120	Two-Way	Enter	Exit	Exit Total		Exit	Total							
Single-Family Detached Housing (210)	50 Homes*	534	10	30	40	33	19	52							

\* - 50 was used for trip generation to conservatively account for changes in land plan or lot count

The directional distribution of the site generated trips was determined based on existing and expected travel patterns in the area. Trip origins and destinations for the proposed development are expected to be 55% to/from the north via Davis Blvd (FM 1938) and 45% to/from east via Davis Blvd (FM 1938). Access to the proposed development will be provided by one full-access roadway on Davis Blvd (FM 1938), Street A. The proposed site plan is shown in **Figure 3**. Future lane assignments and traffic control are shown in Appendix A.



The Trip Distribution Percentages and Site Generated Traffic Volumes are shown in **Appendix A.** The projected traffic volumes for 2027 Background Conditions and 2027 Projected Conditions are shown in **Appendix A.** 







## **Capacity Analysis**

Capacity Analysis was performed using the computer program *Synchro 12*, which is based on the procedures in the *Highway Capacity Manual (HCM)*. Capacity Analysis provides information regarding traffic operations at an intersection and is expressed in terms of the level of service (LOS). The LOS indicates the average seconds of delay experienced by a motorist at a signalized intersection, at stop sign controlled approaches and left turn movements at an unsignalized intersection. Intersection LOS range from A to F, with LOS A representing free flow conditions and LOS F representing highly congested conditions. An intersection operating at or above LOS D is typically characterized by acceptable delays. The Level of Service Measurement and Qualitative Descriptions for Signalized and Unsignalized intersections are shown in **Table 3**.

Level	Unsignalized		Signalized
Of Service	Control Delay Per Vehicle (Sec)	Control Delay Per Vehicle (Sec)	Description
А	≤10	≤ 10	Good progression and short cycle lengths
В	> 10 and ≤ 15	> 10 and ≤ 20	Good progression or short cycle lengths, more vehicle stops
с	> 15 and ≤ 25	> 20 and ≤ 35	Fair progression and/or longer cycle lengths, some cycle failures
D	> 25 and ≤ 35	> 35 and ≤ 55	Congestion becomes noticeable, high volume to capacity ratio
E	> 35 and ≤ 50	> 55 and ≤ 80	Limit of acceptable delay, poor progression, long cycles, and/or high volume
F	> 50	> 80	Unacceptable to drivers, volume greater than capacity

#### Table 3 – Level of Service Measurement and Qualitative Descriptions

The impact of the proposed development at the study area intersections was analyzed using Capacity Analysis for the following scenarios and the *Synchro 12* capacity analysis reports can be found in **Appendix C-E.** 

#### 2024 Existing Conditions

• Existing traffic volumes

#### 2027 Background Conditions

• 2024 Existing Conditions + 2.2% annual background growth rate for three years

#### 2027 Projected Conditions

• 2027 Background Conditions + Full Build Out

**Table 4 & 5** summarize the capacity analysis results for the AM and PM Peak Hours, respectively. All signalized intersections and unsignalized approaches are projected to operate at an acceptable LOS during the Peak Hours except for those in red.



Intersection	2024 E Cond	xisting itions	2027 Ba Cond	ckground itions	2027 Projected Conditions				
	LOS	Sec. of Delay	LOS	Sec. of Delay	LOS	Sec. of Delay			
1. Davis Blvd at Lyndhurst Way/Creek Rd									
Eastbound	E	37.6	E	42.8	E	43.6			
Westbound	F	85.3	F	110.0	F	114.7			
Northbound	С	16.1*	С	17.2*	С	17.3*			
Southbound	D	32.5*	Е	37.4*	Е	38.1*			
2. Davis Blvd at Street A									
Westbound					F	68.2			
Northbound					А	0.0			
Southbound					Е	39.0*			
3. Davis Blvd at Rolling Wood Ln									
Westbound	F	60.4	F	74.0	F	74.8			
Northbound	А	0.0	А	0.0	А	0.0			
Southbound	D	32.2*	E	36.9*	E	37.1*			

#### Table 4 – Capacity Analysis – AM Peak Hour

\*indicates left turn LOS

Intersection	2024 E Cond	xisting itions	2027 Ba Cond	ckground itions	2027 Projected Conditions				
	LOS	Sec. of Delay	LOS	Sec. of Delay	LOS	Sec. of Delay			
1. Davis Blvd at Lyndhurst Way/Creek Rd									
Eastbound	F	179.5	F	278.0	F	291.0			
Westbound	F	97.9	F	149.0	F	154.7			
Northbound	E	42.6*	F	52.1*	F	53.4*			
Southbound	D	25.1*	D	28.0*	D	28.3*			
2. Davis Blvd at Street A									
Westbound					Е	49.7			
Northbound					А	0.0			
Southbound					D	33.9*			
3. Davis Blvd at Rolling Wood Ln									
Westbound	Е	37.2	E	42.5	Е	43.3			
Northbound	А	0.0	А	0.0	А	0.0			
Southbound	С	24.4*	D	27.1*	D	27.6*			

#### Table 5 – Capacity Analysis – PM Peak Hour

\*indicates left turn LOS



For 2027 Background Conditions, at the intersection of Davis Blvd (FM 1938) and Lyndhurst Way, the eastbound, westbound, and southbound approaches are failing with a LOS E or F for the AM Peak Hour, and the eastbound, westbound, and northbound approaches are failing with a LOS E or F for the PM Peak Hour. Although the LOS is failing, LOS E or F is maintained from 2027 Background Conditions to 2027 Projected Conditions for each approach; therefore, no mitigations are recommended.

For 2027 Background Conditions, at the intersection of Davis Blvd (FM 1938) and Rolling Wood Ln, the westbound and southbound approaches are failing with a LOS E or F for the AM Peak Hour, and the westbound approach is failing with a LOS E for the PM Peak Hour. Although the LOS is failing, LOS E or F is maintained from 2027 Background Conditions to 2027 Projected Conditions for each approach; therefore, no mitigations are recommended.

For 2027 Projected Conditions, at the intersection of Davis Blvd (FM 1938) and Street A, the westbound and southbound approaches are failing with a LOS E or F in the AM Peak Hour and the westbound approach is failing with a LOS E in the PM Peak Hour. For the westbound approach, LOS E-F on the minor driveway approach is not uncommon at an unsignalized intersection where traffic volumes on the major roadways are high, such as Davis Blvd (FM 1938). LOS E-F is for the minor roadway approaches only, not the overall intersections, and minimal volumes from the development will be traveling westbound through the intersection (1% of overall traffic at Street A in AM Peak Hour / 0.5% of overall traffic at Street A in PM Peak Hour); therefore, no mitigations are recommended. For the southbound approach, although the LOS is failing in the AM Peak Hour, minimal volumes from the development will be making southbound left turn movements (0.2% of overall traffic at Street A in AM Peak Hour). Additionally, the TxDOT FM 1938 Raised Median Project will eventually restrict the failing southbound left turn movement. Minimal volumes from the development will be making southbound left turn movement. Minimal volumes from the development will be making southbound left turn movement. Minimal volumes from the development will be making southbound left turn movement. Minimal volumes from the development will be making southbound left turn movement. Minimal volumes from the development will be making southbound left turn movements, and future background construction will restrict the southbound left turn movement; therefore, no mitigations are recommended.



#### Right Turn Lane Analysis

Table 2-3 of the *Texas Department of Transportation's (TxDOT) Access Management Manual* provides an Auxiliary Lane Threshold of 60 vehicles per hour for a right turn lane based on a speed limit less than or equal to 45 mph and 50 vehicles per hour based on a speed limit greater than 45 mph. Right turn lane analysis for 2027 Projected Conditions is presented in **Table 6**.

Location (Direction)	Speed Limit	TxDOT Auxiliary Lane Threshold	Right Tur AM Peak Hour	n Volume PM Peak Hour	Minimum Volume Met
Davis Blvd at Street A (Northbound)	50 mph	50 vehicles	5	15	NO

#### Table 6 – 2027 Projected Conditions Right Turn Lane Analysis

The northbound approach at the intersection did not meet the threshold for a right turn lane. Although a right turn lane is required in accordance with TxDOT District Requirements, a northbound right turn lane may not be feasible due to the existing super elevation on Davis Road (FM 1938) and the location of (proposed) Street A is at the southern boundary of the development where any right-of-way necessary would be on an adjacent property; therefore, a northbound right turn lane is not recommended.

#### Driveway Spacing

Table 2-2 of the *TxDOT Access Management Manual* provides a minimum State Highway Connection Spacing (Driveway Spacing) of 425 feet based on speed limits greater than 50 mph. On Davis Blvd (FM 1938), the distance from proposed Street A to the nearest roadway to the north, Creek Road, is 270 feet and 140 feet to the nearest roadway to the south, Rolling Wood Lane. The driveway spacing diagram is shown in **Appendix H.** Minimum driveway spacing requirements are not met because Street A is proposed to align between Rolling Wood Lane and Creek Road. The only available access to the proposed site is along Davis Blvd (FM 1938) between Rolling Wood Lane and Creek Road, and the driveway spacing between Rolling Wood Lane and Creek Road is approximately 525 feet, which does not allow Street A to meet driveway spacing requirements; therefore, a variance is requested for Street A.

#### Stopping Sight Distance

Table 3-1 of the American Association of State Highway and Transportation Officials' (AASHTO) Green Book provides a Stopping Sight Distance of 425 feet based on a speed limit of 50 mph and a level roadway. The northbound and southbound movements on Davis Blvd (FM 1938) are unobstructed and on a level roadway which meets the minimum 425 foot Stopping Sight Distance. The roadway has been field verified and site visit photos are located in **Appendix I**.

#### Intersection Sight Distance

Table 9-6 and Table 9-8 of the AASHTO Green Book provide Intersection Sight Distances for Case B1, Left Turn from Stop, and Case B2, Right Turn from Stop, respectively. The minimum Intersection Sight Distance for Case B1, Left Turn from Stop, is 670 feet. The minimum Intersection Sight Distance for Case B2, Right Turn from Stop, is 480 feet. Both turning movements at the proposed roadway, Street A, are unobstructed. The Sight Distance Triangles Diagram and site visit photos are located in **Appendix J**.



#### **Recommendations**

Traffic impacts to the surrounding roadway are minimal and approval is recommended with the following recommendations:

1. Request a variance for driveway spacing for proposed Street A as part of the permit process.

## **Appendix**

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- Appendix A Lane Assignment and Traffic Volumes
- Appendix B Traffic Data
- Appendix C Capacity Analysis 2024 Existing Conditions
- Appendix D Capacity Analysis 2027 Background Conditions
- Appendix E Capacity Analysis 2027 Projected Conditions
- Appendix F Trip Generation Reports
- Appendix G FM 1938 Raised Median Project Fact Sheet and Schematic
- Appendix H Driveway Spacing Diagram
- Appendix I Stopping Sight Distance Photos
- Appendix J Intersection Sight Distance Triangle Diagram and Photos

Appendix A Lane Assignment and Traffic Volumes









2024 Existing Conditions AM Peak Hour





2024 Existing Conditions PM Peak Hour



12/05/2024 Quiddity Engineering

2027 Background Conditions AM Peak Hour





2027 Background Conditions PM Peak Hour





Armstrong Hills North ^

Inbound Trip Distribution





Armstrong Hills North ^

Outbound Trip Distribution



12/05/2024 Quiddity Engineering

Site Generated Volumes AM Peak Hour





Site Generated Volumes PM Peak Hour



2027 Projected Conditions AM Peak Hour



12/05/2024 Quiddity Engineering

2027 Projected Conditions PM Peak Hour



12/05/2024 Quiddity Engineering

Appendix B Traffic Data

#### 1. FM 1938 at Lyndhurst Way/Creek Road - TMC

Wed Oct 23, 2024

Full Length (6 AM-9 AM, 4 PM-7 PM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

All Movements

ID: 1239614, Location: 32.917463, -97.186597



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave., Pasadena, TX, 77503, US

Leg	FM 193	38					FM 193	8					Lyndhu	ırst Wa	ау				Creek R	oad					
Direction	Northb	ound					Southbo	ound					Eastbou	und					Westbou	ind					i.
Time	L	Т	R	U	App I	Ped*	L	Т	R	U	Арр	Ped*	L	Т	R	U	App 1	Ped*	L	Т	R	U	App P	ed*	Int
2024-10-23 6:00AM	0	143	0	0	143	0	1	80	0	0	81	0	2	0	0	0	2	0	1	1	0	0	2	0	228
6:15AM	0	197	0	0	197	0	0	89	0	0	89	0	0	0	1	0	1	0	0	0	2	0	2	0	289
6:30AM	2	271	0	0	273	0	1	112	0	0	113	0	3	0	2	0	5	0	0	0	0	0	0	0	391
6:45AM	0	346	0	0	346	0	0	148	2	0	150	0	6	0	1	0	7	0	0	0	0	0	0	0	503
Hourly Total	2	957	0	0	959	0	2	429	2	0	433	0	11	0	4	0	15	0	1	1	2	0	4	0	1411
7:00AM	3	352	0	0	355	0	1	185	1	0	187	0	1	0	3	0	4	0	0	0	1	0	1	0	547
7:15AM	1	404	1	0	406	0	0	215	4	0	219	0	2	0	2	0	4	0	1	1	1	0	3	0	632
7:30AM	1	472	1	0	474	0	2	255	0	0	257	0	2	0	3	0	5	0	0	0	3	0	3	0	739
7:45AM	1	478	1	0	480	0	0	287	3	0	290	0	0	0	3	0	3	0	1	0	3	0	4	0	777
Hourly Total	6	1706	3	0	1715	0	3	942	8	0	953	0	5	0	11	0	16	0	2	1	8	0	11	0	2695
8:00AM	2	426	0	0	428	0	3	238	1	0	242	0	0	0	2	0	2	0	2	0	2	0	4	0	676
8:15AM	2	412	1	0	415	0	0	212	1	0	213	0	4	1	2	0	7	0	3	0	1	0	4	0	639
8:30AM	1	408	0	0	409	0	0	231	2	0	233	0	7	0	2	0	9	0	4	0	1	0	5	0	656
8:45AM	3	411	2	0	416	0	0	238	4	0	242	0	1	0	2	0	3	0	1	0	0	0	1	0	662
Hourly Total	8	1657	3	0	1668	0	3	919	8	0	930	0	12	1	8	0	21	0	10	0	4	0	14	0	2633
4:00PM	3	319	2	0	324	0	0	439	4	0	443	0	4	0	3	0	7	0	0	1	2	0	3	0	777
4:15PM	0	333	2	0	335	0	1	467	5	0	473	0	5	0	4	0	9	0	2	0	1	0	3	0	820
4:30PM	2	350	2	0	354	0	4	459	4	0	467	0	1	0	4	0	5	0	0	0	3	0	3	0	829
4:45PM	4	397	1	0	402	0	0	492	2	0	494	0	3	0	5	0	8	0	3	0	1	0	4	0	908
Hourly Total	9	1399	7	0	1415	0	5	1857	15	0	1877	0	13	0	16	0	29	0	5	1	7	0	13	0	3334
5:00PM	2	408	5	1	416	0	1	472	0	0	473	0	0	0	1	0	1	0	3	1	2	0	6	0	896
5:15PM	2	400	1	0	403	0	0	518	3	0	521	0	5	1	3	0	9	0	0	0	1	0	1	0	934
5:30PM	1	393	2	1	397	0	3	516	3	0	522	0	2	1	2	0	5	0	2	1	1	0	4	0	928
5:45PM	6	329	2	0	337	0	2	496	3	0	501	0	3	0	5	0	8	0	2	0	1	0	3	0	849
Hourly Total	11	1530	10	2	1553	0	6	2002	9	0	2017	0	10	2	11	0	23	0	7	2	5	0	14	0	3607
6:00PM	5	330	0	0	335	0	2	380	3	0	385	0	2	0	6	1	9	0	1	1	2	0	4	0	733
6:15PM	2	314	1	0	317	0	1	416	2	0	419	0	3	0	2	0	5	0	1	0	0	0	1	0	742
6:30PM	3	307	2	0	312	0	1	344	4	0	349	0	5	0	2	0	7	0	0	1	1	0	2	0	670
6:45PM	5	270	0	0	275	0	1	322	2	0	325	0	1	0	1	0	2	0	1	0	4	0	5	0	607
Hourly Total	15	1221	3	0	1239	0	5	1462	11	0	1478	0	11	0	11	1	23	0	3	2	7	0	12	0	2752
Total	51	8470	26	2	8549	0	24	7611	53	0	7688	0	62	3	61	1	127	0	28	7	33	0	68	0	16432
% Approach	0.6%	99.1%	0.3%	0%	-	_	0.3%	99.0%	0.7%	0%	-	-	48.8%	2.4%	48.0%	0.8%	-	-	41.2% 1	0.3%	48.5% 0	)%	-	-	-
% Total	0.3%	51.5%	0.2%	0%	52.0%	-	0.1%	46.3%	0.3%	0%	46.8%	-	0.4%	0%	0.4%	0%	0.8%	-	0.2%	0%	0.2% 0	)%	0.4%	-	-
Lights	50	8364	24	2	8440	-	23	7516	52	0	7591	-	62	3	59	1	125	-	28	6	30	0	64	-	16220
% Lights	98.0%	98 7%	92.3%	100%	98 7%	-	95.8%	38.8%	98 1%	0%	98 7%	-	100%	100%	96 7%	100%	98 4%	-	100% 8	5 7%	90.9% (	)% <b>c</b>	4 1%	-	98 7%
Articulated Trucks	0	17	0	0	17	-	0	14	0	0	14	-	0	0	0	0	0	-	0	0	0	0	0	-	31
% Articulated Trucks	0%	0.2%	0%	0%	0.2%	-	0%	0.2%	0%	0%	0.2%	-	0%	0%	0%	0%	0%	-	0%	0%	0% (	)%	0%	-	0.2%
Buses and Single-Unit																									
Trucks	1	89	2	0	92	-	1	81	1	0	83	-	0	0	2	0	2	-	0	1	3	0	4	-	181
% Buses and Single-Unit																									
Trucks	2.0%	1.1%	7.7%	0%	1.1%	-	4.2%	1.1%	1.9%	0%	1.1%	-	0%	0%	3.3%	0%	1.6%	-	0% 1	4.3%	9.1% 0	)%	5.9%	-	1.1%
Pedestrians	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Bicycles on Crosswalk	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-

#### 1. FM 1938 at Lyndhurst Way/Creek Road - TMC

Wed Oct 23, 2024 AM Peak (7:30 AM - 8:30 AM)

All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk)

All Movements

ID: 1239614, Location: 32.917463, -97.186597



Provided by: C. J. Hensch & Associates Inc.

5215 Sycamore Ave., Pasadena, TX, 77503, US

Leg	FM 19	938					FM 19	938					Lyndhu	ırst W	ay				Creek F	load	i				
Direction	North	bound					South	bound					Eastbo	und					Westbo	und					
Time	L	Т	R	U	Арр	Ped*	L	Т	R	U	Арр	Ped*	L	Т	R	U	App	Ped*	L	Т	R	U	App P	ed*	Int
2024-10-23 7:30AM	1	472	1	0	474	0	2	255	0	0	257	0	2	0	3	0	5	0	0	0	3	0	3	0	739
7:45AM	1	478	1	0	480	0	0	287	3	0	290	0	0	0	3	0	3	0	1	0	3	0	4	0	777
8:00AM	2	426	0	0	428	0	3	238	1	0	242	0	0	0	2	0	2	0	2	0	2	0	4	0	676
8:15AM	2	412	1	0	415	0	0	212	1	0	213	0	4	1	2	0	7	0	3	0	1	0	4	0	639
Total	6	1788	3	0	1797	0	5	992	5	0	1002	0	6	1	10	0	17	0	6	0	9	0	15	0	2831
% Approach	0.3%	99.5%	0.2%	0%	-	-	0.5%	99.0%	0.5%	0%	-		35.3%	5.9%	58.8%	0%	-	-	40.0%	0% (	50.0% (	)%	-	-	-
% Total	0.2%	63.2%	0.1%	0%	63.5%	-	0.2%	35.0%	0.2%	0%	35.4%	-	0.2%	0%	0.4%	0%	0.6%	-	0.2%	0%	0.3% (	)%	0.5%	-	-
PHF	0.750	0.935	0.750	-	0.936	-	0.417	0.864	0.417	-	0.864	-	0.375	0.250	0.833	- (	0.607	-	0.500	-	0.750	- (	0.938	-	0.911
Lights	6	1751	3	0	1760	-	5	978	5	0	988	-	6	1	10	0	17	-	6	0	9	0	15	-	2780
% Lights	100%	97.9%	100%	0%	97.9%	-	100%	98.6%	100%	0%	98.6%	-	100%	100%	100%	0% 1	100%	-	100%	0%	100% (	)% 1	100%	-	98.2%
Articulated Trucks	0	9	0	0	9	-	0	1	0	0	1	-	0	0	0	0	0	-	0	0	0	0	0	-	10
% Articulated Trucks	0%	0.5%	0%	0%	0.5%	-	0%	0.1%	0%	0%	0.1%	-	0%	0%	0%	0%	0%	-	0%	0%	0% (	)%	0%	-	0.4%
Buses and Single-Unit Trucks	0	28	0	0	28	-	0	13	0	0	13	-	0	0	0	0	0	-	0	0	0	0	0	_	41
% Buses and Single-Unit Trucks	0%	1.6%	0%	0%	1.6%	-	0%	1.3%	0%	0%	1.3%	-	0%	0%	0%	0%	0%	-	0%	0%	0% (	)%	0%	_	1.4%
Pedestrians	-	-	_	-	-	0	-	-	-	-	-	. 0	-	-	-	-	-	0	-	-	-	-	-	0	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-		-	-	-	-	-	-	-	-	-	-	-	-	-
Bicycles on Crosswalk	-	-	-	-	-	0	-	-	-	-	-	· 0	-	-	-	-	-	0	-	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	_	-	-	-	-	-	-	-	_		-	-	-	-	_	_	-			-		-	-

#### 1. FM 1938 at Lyndhurst Way/Creek Road - TMC

Wed Oct 23, 2024 PM Peak (4:45 PM - 5:45 PM) - Overall Peak Hour All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk) All Movements ID: 1239614, Location: 32.917463, -97.186597



Provided by: C. J. Hensch & Associates Inc.

> 5215 Sycamore Ave., Pasadena, TX, 77503, US

Leg	FM 19	938					FM 19	938					Lyndhu	ırst Wa	ay				Creek I	Road					
Direction	North	bound					South	oound					Eastbo	und					Westbo	ound					
Time	L	Т	R	U	App P	ed*	L	Т	R	U	Арр	Ped*	L	Т	R	U	App I	ed*	L	Т	R	U	App I	Ped*	Int
2024-10-23 4:45PM	4	397	1	0	402	0	0	492	2	0	494	0	3	0	5	0	8	0	3	0	1	0	4	0	908
5:00PM	2	408	5	1	416	0	1	472	0	0	473	0	0	0	1	0	1	0	3	1	2	0	6	0	896
5:15PM	2	400	1	0	403	0	0	518	3	0	521	0	5	1	3	0	9	0	0	0	1	0	1	0	934
5:30PM	1	393	2	1	397	0	3	516	3	0	522	0	2	1	2	0	5	0	2	1	1	0	4	0	928
Total	9	1598	9	2	1618	0	4	1998	8	0	2010	0	10	2	11	0	23	0	8	2	5	0	15	0	3666
% Approach	0.6%	98.8%	0.6%	0.1%	-	-	0.2%	99.4%	0.4% (	)%	-	-	43.5%	8.7%	47.8%	0%	-	-	53.3%	13.3%	33.3%	0%	-	-	-
% Total	0.2%	43.6%	0.2%	0.1%	44.1%	-	0.1%	54.5%	0.2% (	)% :	54.8%	-	0.3%	0.1%	0.3%	0%	0.6%	-	0.2%	0.1%	0.1%	0%	0.4%	-	-
PHF	0.563	0.979	0.450	0.500	0.972	-	0.333	0.964	0.667	-	0.963	-	0.500	0.500	0.550	-	0.639	-	0.667	0.500	0.625	-	0.625	-	0.981
Lights	9	1585	8	2	1604	-	4	1972	8	0	1984	-	10	2	11	0	23	-	8	2	4	0	14	-	3625
% Lights	100%	99.2%	88.9%	100%	99.1%	-	100%	98.7%	100% (	)% 9	98.7%	-	100%	100%	100%	0%	100%	-	100%	100%	80.0%	0% 9	93.3%	-	98.9%
Articulated Trucks	0	2	0	0	2	-	0	4	0	0	4	_	0	0	0	0	0	-	0	0	0	0	0	-	6
% Articulated Trucks	0%	0.1%	0%	0%	0.1%	-	0%	0.2%	0% (	)%	0.2%	-	0%	0%	0%	0%	0%	-	0%	0%	0%	0%	0%	-	0.2%
Buses and Single-Unit Trucks	0	11	1	0	12	_	0	22	0	0	22	_	0	0	0	0	0	_	0	0	1	0	1	_	35
% Buses and Single-Unit			-	-			-		-				-	-	-		-		-	-		-			
Trucks	0%	0.7%	11.1%	0%	0.7%	-	0%	1.1%	0% (	)%	1.1%	-	0%	0%	0%	0%	0%	-	0%	0%	20.0%	0%	6.7%	-	1.0%
Pedestrians	-	_	_	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Bicycles on Crosswalk		-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	-	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	_	-	-	-	-	-	-	-	-	-	-	-	-	-

#### 2. FM 1938 at Rolling Wood Lane - TMC

Wed Oct 23, 2024 Full Length (6 AM-9 AM, 4 PM-7 PM) All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk) All Movements ID: 1239615, Location: 32.91594, -97.186481



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave.,

Pasadena, TX, 77503, US

Leg	FM 1938 FM 1938										Rolling Wood Lane					
Direction	Northbou	nd				Southbound				Westbound						
Time	Т	R	U	Арр	Ped*	L	Т	U	Арр	Ped*	L	R	U	Арр	Ped*	Int
2024-10-23 6:00AM	141	0	0	141	0	0	82	0	82	0	0	1	0	1	0	224
6:15AM	187	0	0	187	0	0	99	0	99	0	0	0	0	0	0	286
6:30AM	267	1	0	268	0	0	109	0	109	0	0	2	0	2	0	379
6:45AM	357	1	0	358	0	0	141	0	141	0	1	0	0	1	0	500
Hourly Total	952	2	0	954	0	0	431	0	431	0	1	3	0	4	0	1389
7:00AM	345	0	0	345	0	0	203	0	203	0	0	2	0	2	0	550
7:15AM	392	0	0	392	0	0	200	0	200	0	0	0	0	0	0	592
7:30AM	478	1	0	479	0	1	252	1	254	0	5	4	0	9	0	742
7:45AM	465	1	0	466	0	0	303	0	303	0	1	0	0	1	0	770
Hourly Total	1680	2	0	1682	0	1	958	1	960	0	6	6	0	12	0	2654
8:00AM	435	1	0	436	0	2	230	0	232	0	1	4	0	5	0	673
8:15AM	409	1	0	410	0	0	230	0	230	0	0	2	0	2	0	642
8:30AM	400	1	0	401	0	0	245	0	245	0	1	0	0	1	0	647
8:45AM	409	0	0	409	0	0	239	0	239	0	1	0	0	1	0	649
Hourly Total	1653	3	0	1656	0	2	944	0	946	0	3	6	0	9	0	2611
4:00PM	327	2	0	329	0	0	433	0	433	0	1	1	0	2	0	764
4:15PM	317	3	0	320	0	1	493	0	494	0	2	1	0	3	0	817
4:30PM	360	0	0	360	0	1	466	0	467	0	2	1	0	3	0	830
4:45PM	404	2	0	406	0	0	501	0	501	0	1	1	0	2	0	909
Hourly Total	1408	7	0	1415	0	2	1893	0	1895	0	6	4	0	10	0	3320
5:00PM	414	0	1	415	0	1	488	0	489	0	0	1	0	1	0	905
5:15PM	396	1	0	397	0	1	505	0	506	0	1	0	0	1	0	904
5:30PM	390	2	0	392	0	0	524	0	524	0	0	0	0	0	0	916
5:45PM	341	0	1	342	0	1	507	0	508	0	0	1	0	1	0	851
Hourly Total	1541	3	2	1546	0	3	2024	0	2027	0	1	2	0	3	0	3576
6:00PM	317	2	1	320	0	0	400	0	400	0	0	0	0	0	0	720
6:15PM	320	2	0	322	0	1	413	0	414	0	0	0	0	0	0	736
6:30PM	318	1	0	319	0	1	352	1	354	0	1	0	0	1	0	674
6:45PM	259	0	0	259	0	0	320	0	320	0	2	0	0	2	0	581
Hourly Total	1214	5	1	1220	0	2	1485	1	1488	0	3	0	0	3	0	2711
Total	8448	22	3	8473	0	10	7735	2	7747	0	20	21	0	41	0	16261
% Approach	99.7%	0.3%	0%	-	-	0.1%	99.8%	0%	-	-	48.8%	51.2%	0%	-	-	-
% Total	52.0%	0.1%	0%	52.1%	-	0.1%	47.6%	0%	47.6%	-	0.1%	0.1%	0%	0.3%	-	-
Lights	8337	21	3	8361	-	10	7657	2	7669	-	20	20	0	40	-	16070
% Lights	98.7%	95.5%	100%	98.7%	-	100%	99.0%	100%	99.0%	-	100%	95.2%	0%	97.6%	-	98.8%
Articulated Trucks	23	0	0	23	-	0	13	0	13	-	0	0	0	0	-	36
% Articulated Trucks	0.3%	0%	0%	0.3%	-	0%	0.2%	0%	0.2%	-	0%	0%	0%	0%	-	0.2%
Buses and Single-Unit Trucks	88	1	0	89	-	0	65	0	65	-	0	1	0	1	-	155
% Buses and Single-Unit Trucks	1.0%	4.5%	0%	1.1%	-	0%	0.8%	0%	0.8%	-	0%	4.8%	0%	2.4%	-	1.0%
Pedestrians	-	-	-	-	0	-	-	-	-	0	-	-	-	-	0	
% Pedestrians	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
Bicycles on Crosswalk	-	-	-	-	0	-	-	-	-	0	-	-	-	-	0	
% Bicycles on Crosswalk	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-	-
## 2. FM 1938 at Rolling Wood Lane - TMC

Wed Oct 23, 2024 AM Peak (7:30 AM - 8:30 AM) All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk) All Movements ID: 1239615, Location: 32.91594, -97.186481



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave., Pasadena, TX, 77503, US

FM 1938 FM 1938 Rolling Wood Lane Leg Direction Northbound Southbound Westbound Time Т R U Ped\* L Т U Арр Ped\* L R U Арр Ped\* Int Арр 2024-10-23 7:30AM 478 0 252 5 0 742 1 479 0 1 1 254 0 4 9 0 0 0 770 7:45AM 465 0 466 0 303 0 303 0 1 1 0 0 1 8:00AM 435 1 0 436 0 2 230 0 232 0 1 4 0 5 0 673 8:15AM 0 230 0 409 1 0 410 0 0 230 0 2 0 2 0 642 Total 1787 4 1791 0 3 1015 1 1019 0 7 10 0 17 0 2827 0 % Approach 99.8% 0.2% 0% 0.3% 99.6% 0.1% 41.2% 58.8% 0% % Total 63.2% 0.1% 0% 63.4% 0.1% 35.9% 0% 36.0% 0.2% 0.4% 0% 0.6% PHF 0.250 0.935 1.000 0.935 0.375 0.837 0.841 0.350 0.625 0.472 0.918 --Lights 1746 4 0 1750 3 1003 1007 7 10 0 17 2774 1 % Lights 97.7% 100% 0% 97.7% 100% 98.8% 100% 98.8% 100% 100% 0% 100% 98.1% Articulated Trucks 8 0 0 8 0 1 0 1 0 0 0 0 9 % Articulated Trucks 0.4% 0% 0.3% 0% 0% 0.4% 0.1% 0% 0.1% 0% 0% 0% 0% **Buses and Single-Unit Trucks** 0 33 0 0 33 0 0 0 0 0 44 1111 % Buses and Single-Unit Trucks 1.8% 0% 0% 1.8% 0% 1.1%0% 1.1% 0% 0% 0% 0% 1.6% Pedestrians 0 0 0 \_ % Pedestrians \_ -\_ \_ \_ \_ \_ \_ \_ \_ Bicycles on Crosswalk 0 0 0 % Bicycles on Crosswalk \_ --\_ \_ ---\_ \_ -\_

Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

## 2. FM 1938 at Rolling Wood Lane - TMC

Wed Oct 23, 2024 PM Peak (4:45 PM - 5:45 PM) - Overall Peak Hour All Classes (Lights, Articulated Trucks, Buses and Single-Unit Trucks, Pedestrians, Bicycles on Crosswalk) All Movements ID: 1239615, Location: 32.91594, -97.186481



Provided by: C. J. Hensch & Associates Inc. 5215 Sycamore Ave., Pasadena, TX, 77503, US

FM 1938 FM 1938 Rolling Wood Lane Leg Direction Northbound Southbound Westbound Time Т R U Арр Ped\* L Т U Ped\* L R U Арр Ped\* Int Арр 2024-10-23 4:45PM 404 2 0 0 501 0 0 909 406 0 501 0 1 1 2 0 0 488 0 5:00PM 414 415 0 0 489 0 0 0 905 1 1 1 1 5:15PM 396 1 0 397 0 1 505 0 506 0 1 0 0 1 0 904 5:30PM 0 0 390 2 0 392 0 524 0 524 0 0 0 0 0 916 Total 1604 5 1 1610 0 2 2018 2020 0 2 2 0 4 0 3634 0 % Approach 99.6% 0.3% 0.1% 0.1% 99.9% 0% 50.0% 50.0% 0% % Total 44.1% 0.1% 0% 44.3% 0.1% 55.5% 0% 55.6% 0.1% 0.1% 0% 0.1% 0.250 PHF 0.969 0.625 0.970 0.500 0.963 0.964 0.500 0.500 0.500 0.992 --Lights 1589 4 1594 2 2002 0 2004 2 1 0 3 3601 1 % Lights 99.1% 80.0% 100% 99.0% 100% 99.2% 0% 99.2% 100% 50.0% 0% 75.0% 99.1% Articulated Trucks 2 0 0 2 0 3 0 3 0 0 0 0 5 % Articulated Trucks 0.1% 0% 0% 0.1% 0% 0.1% 0% 0.1% 0% 0% 0% 0% 0.1% Buses and Single-Unit Trucks 0 0 0 0 0 28 13 1 14 13 13 1 1 % Buses and Single-Unit Trucks 0.8% 20.0% 0% 0.9% 0% 0.6% 0% 0.6% 0% 50.0% 0% 25.0% 0.8% Pedestrians 0 0 0 \_ % Pedestrians \_ \_ \_ \_ \_ \_ \_ \_ -Bicycles on Crosswalk 0 0 0 % Bicycles on Crosswalk -\_ ------\_

Pedestrians and Bicycles on Crosswalk. L: Left, R: Right, T: Thru, U: U-Turn

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Location ID	22003748					M	POID	282	
Туре	SPOT					HP	MSID	UNASSIC	GNED
SF Group	FORT WORT	H FC 3 (2024	)		•	Route	Туре	FM	
AF Group	HP897 (2024	)			•	F	Route	1938	
GF Group	- FC - (2014)				•	1	ctive	Yes	
ass Dist Grp	)					Cat	egory	URBAN A	ACR
eas Clss Grp									
WIM Group	2						1		
QC Group	Default								
Fnct'l Class	(3)Principal A	rterial-Other					j)		
Located Or	FM1938								
Loc On Alias	FM1938-KG								
ore Detail 🕨									
TATION DA	TA								
irections:	2-WAY NB	SB 🔒				_			
AADT 🕐									
Yea	r AADT	DHV-30	Κ%	D %	P/	۸ I	1	BC	Src
201	9 37,239	3,527	9	58	36,825	(99%)	414	(1%)	
201	4 29.463	3.436	12	57			-		

	2019	31,239	3,321	9	30	30,0	23 (3370)	414(	170)	
	2014	29,463	3,436	12	57					
rave	el Demano	1 Model								
	Model Year	Model AADT	AM PHV	AM PPV	MD PHV	MD PPV	PM PHV	PM PPV	NT PHV	NT PP
OLI	JME COU	NT				VOLUM	AF TREN	n 🕐		
		Date		Int	Total	Year		Annu	al Growth	1
2	M	on 4/22/2019	)	15	38,729	2024		_	-1%	
5	M	on 4/14/2014	4	15	38,365	2019	,		5%	
PEE	Đ					CLASS	FICATIO	DN		
	Date	Int F	ace	85th	Total		Date	Int		Total
		No	Data					No Data		
/EIG	H-IN-MO	TION (2)				PER V	EHICLE			
T	Date	Axles	Avg G	w	Total		Date	Axies	85th	Total
_		No	Data		1.1			No Data		
AP	8									
	Date		Int	Tot	al					
_		No	Data	10.52						
DAR	TIAL COL	INT								
T	Date	Int	24	Hr Total						
OTI	SIEILES				-					
011	SITILES			Note	_			0	ate	
										÷



List	View	1 4	II DIRs									Re	port Center
0	Record	144		1		ы	of 1 G	ioto	Record		00		
Loos	tion ID.	2201120	16						11111		IDO ID	282	
Loca	Type	SPOT	10							HD	MSID	LINASSIC	NED
SF	Group	FORT	NORT	H FC 3 (2	2024)				•	Route	Type	UNASSIC	INC.D
AF	Group	HP897	(2024	)		_		_			Route		
GF	Group	FORT	NORT	H FC - (2	015)	_		_	÷		Active	Yes	
ass D	Dist Grp				2024					Cat	enory	URBANA	CR
eas Cl	Iss Grp	FORT	NORT	H (2016)	ξ.	_			•				1000
WIM	Group												
QC	Group	Volume	Group	p 4									
Fnct	'l Class	(3)Princ	cipal A	rterial-Otl	her						, i		
Loca	sted On	-											
.00 0	n Allas												
re De	tail 🕨	_											
TATI		<b>A</b>											
irecti	ions'	A NAV	ND	CD 6									
necu	10113.	2-11/41	ND	30	<u> </u>								
AAD	т 🕐												
	Year	AA	DT	DHV-3	0	K% D%		%	PA		1	BC	Src
_	2022	32,	186 <sup>3</sup>		_			_	22 762 (005/)				PBV
	2019	33,	154	3,02	1	9	6	1	32,763 (99%)		391	(1%)	
_	2014	36,	464	3,280	5	9	5	8		_	-	-	_
_	2009	33,	860			_		_		_			
rave	I Demai	nd Moo	lel	-	-		4		- 1		_	-	-
	Year	A	ADT	AM PH	IV AM	PPV	MD PH	V M	DPPV	PM PHV	PM PP	V NT PH	V NT PPV
OLU	ME CO	UNT		- 14						E TREN	0		
		Dat	le		Int	82	Total		Year		Ann	ual Grow	th
	1	Mon 4/2	2/2019	)	15	+	34,486		2024			6%	
5	_	Tue 4/1	/2014	_	15	-	37,983		2022	-0		-1%	
									2019			-2%	
							I ALLEL	۹L	2014			1%	
PEE	D							0	CLASS	FICATIO	N		
	Date	Int	P	ace	85th	201	Total			Date	h	nt	Total
			No	Data	_						No Da	ta	
VEIG	H-IN-M	DTION	0					F	PER VE	HICLE			
	Date	Axi	es	Avg	GVW		Total		0	)ate	Axles	85th	Total
			No	Data							No Da	13	
SAP													
	De	ste		Int		To	tal						
- 0.2			No	Data		_							
PART	TIAL CO	DUNT	102										
	Date		nt	1	24-Hr	Total							
IOTE	S/FILE	S											
					1	Note					1	Date	



Appendix C Capacity Analysis – 2024 Existing Conditions 1.2

## Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ŧ	1		\$		۲	<b>**i</b>		۲	<b>**</b>	
Traffic Vol, veh/h	6	1	10	6	0	9	6	1788	3	5	992	5
Future Vol, veh/h	6	1	10	6	0	9	6	1788	3	5	992	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	0	-	-	-	100	-	-	100	-	-
Veh in Median Storage	, # -	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	38	25	83	50	92	75	75	94	75	42	86	42
Heavy Vehicles, %	0	0	0	0	0	0	0	2	0	0	1	0
Mvmt Flow	16	4	12	12	0	12	8	1902	4	12	1153	12

Major/Minor	Minor2		Ν	Minor1		ľ	Major1		Ν	/lajor2			
Conflicting Flow All	1960	3105	583	2407	3109	953	1165	0	0	1906	0	0	
Stage 1	1183	1183	-	1920	1920	-	-	-	-	-	-	-	
Stage 2	777	1922	-	487	1189	-	-	-	-	-	-	-	
Critical Hdwy	6.4	6.5	7.1	6.4	6.5	7.1	5.3	-	-	5.3	-	-	
Critical Hdwy Stg 1	7.3	5.5	-	7.3	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.7	5.5	-	6.7	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.8	4	3.9	3.8	4	3.9	3.1	-	-	3.1	-	-	
Pot Cap-1 Maneuver	69	12	394	36	12	226	331	-	-	143	-	-	
Stage 1	151	265	-	45	116	-	-	-	-	-	-	-	
Stage 2	327	116	-	489	264	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	58	11	394	30	10	226	331	-	-	143	-	-	
Mov Cap-2 Maneuver	107	69	-	40	74	-	-	-	-	-	-	-	
Stage 1	138	243	-	44	113	-	-	-	-	-	-	-	
Stage 2	302	113	-	428	242	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Ctrl Dly, s/v	37.56	85.27	0.07	0.33	
HCM LOS	E	F			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2V	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	331	-	-	97	394	67	143	-	-
HCM Lane V/C Ratio	0.024	-	-	0.205	0.031	0.356	0.083	-	-
HCM Ctrl Dly (s/v)	16.1	-	-	51.7	14.4	85.3	32.5	-	-
HCM Lane LOS	С	-	-	F	В	F	D	-	-
HCM 95th %tile Q(veh)	0.1	-	-	0.7	0.1	1.3	0.3	-	-

0.8					
WBL	WBR	NBT	NBR	SBL	SBT
۲.	1	<b>**</b>		٦	***
7	10	1787	4	4	1015
7	10	1787	4	4	1015
0	0	0	0	0	0
Stop	Stop	Free	Free	Free	Free
-	None	-	None	-	None
0	0	-	-	100	-
,# 1	-	0	-	-	0
0	-	0	-	-	0
35	63	94	100	38	84
0	0	2	0	0	1
20	16	1901	4	11	1208
	0.8 WBL 7 7 0 Stop - 0 , # 1 0 35 0 20	0.8 WBL WBR 7 10 7 10 7 00 5top Stop 5top Stop 0 0 ,# 1 - 0 - 35 63 0 0 20 16	0.8 WBL WBR NBT 7 10 1787 7 10 1787 7 10 1787 0 0 0 Stop Stop Free None - None - 0 0 - ,# 1 - 0 0 35 63 94 0 2 20 16 1901	0.8         NBT         NBR           WBL         WBR         NBT         NBR           1         1787         4           7         10         1787         4           7         10         1787         4           0         0         0         0           Stop         Stop         Free         Free           0         0         -         0           0         0         -         -           0         0         -         -           0         0         -         -           0         0         -         -           1         -         0         -           35         63         94         100           0         0         2         0           20         16         1901         4	0.8       NBR       NBR       SBL         WBL       WBR       NBT       NBR       SBL         T       ↑↑↑↓       NBR       SBL         T       ↑↑↑↓       NBR       SBL         T       ↑↑↑↓       NBR       SBL         T       ↑↑↑↓↓       ↓       ↑         T       100       1787       4       4         O       0       0       0       0         Stop       Stop       Free       Free       Free         None       -       None       -         O       0       -       100       -         ,#       1       -       0       -       -         O       -       0       -       -       -         35       63       94       100       38       -         Q       0       2       0       0       -         Q       16       1901       4       11

Major/Minor	Minor1	М	ajor1	Ν	lajor2			
Conflicting Flow All	2407	953	0	0	1905	0		
Stage 1	1903	-	-	-	-	-		
Stage 2	504	-	-	-	-	-		
Critical Hdwy	5.7	7.1	-	-	5.3	-		
Critical Hdwy Stg 1	6.6	-	-	-	-	-		
Critical Hdwy Stg 2	6	-	-	-	-	-		
Follow-up Hdwy	3.8	3.9	-	-	3.1	-		
Pot Cap-1 Maneuver	58	226	-	-	143	-		
Stage 1	67	-	-	-	-	-		
Stage 2	527	-	-	-	-	-		
Platoon blocked, %			-	-		-		
Mov Cap-1 Maneuve	r 54	226	-	-	143	-		
Mov Cap-2 Maneuve	r 61	-	-	-	-	-		
Stage 1	67	-	-	-	-	-		
Stage 2	488	-	-	-	-	-		

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	60.37	0	0.28
HCM LOS	F		

Minor Lane/Major Mvmt	NBT	NBRWBLn1V	/BLn2	SBL	SBT	
Capacity (veh/h)	-	- 61	226	143	-	
HCM Lane V/C Ratio	-	- 0.329	0.07	0.074	-	
HCM Ctrl Dly (s/v)	-	- 90.7	22.1	32.2	-	
HCM Lane LOS	-	- F	С	D	-	
HCM 95th %tile Q(veh)	-	- 1.2	0.2	0.2	-	

3

## Intersection

Int Delay, s/veh

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ŧ	1		\$		۲.	<b>***</b>		۲	<b>***</b>	
Traffic Vol, veh/h	10	2	11	8	2	5	11	1598	9	4	1998	8
Future Vol, veh/h	10	2	11	8	2	5	11	1598	9	4	1998	8
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	0	-	-	-	100	-	-	100	-	-
Veh in Median Storage,	# -	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	50	50	55	67	50	63	56	98	45	33	96	67
Heavy Vehicles, %	0	0	0	0	0	20	0	1	11	0	1	0
Mvmt Flow	20	4	20	12	4	8	20	1631	20	12	2081	12

Major/Minor	Minor2		ľ	Minor1		N	Major1		N	lajor2			
Conflicting Flow All	2805	3801	1047	2539	3797	825	2093	0	0	1651	0	0	
Stage 1	2111	2111	-	1680	1680	-	-	-	-	-	-	-	
Stage 2	694	1690	-	859	2117	-	-	-	-	-	-	-	
Critical Hdwy	6.4	6.5	7.1	6.4	6.5	7.5	5.3	-	-	5.3	-	-	
Critical Hdwy Stg 1	7.3	5.5	-	7.3	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.7	5.5	-	6.7	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.8	4	3.9	3.8	4	4.1	3.1	-	-	3.1	-	-	
Pot Cap-1 Maneuver	20	4	196	30	4	243	115	-	-	192	-	-	
Stage 1	33	93	-	67	153	-	-	-	-	-	-	-	
Stage 2	368	151	-	291	92	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	~ 14	~ 3	196	19	~ 3	243	115	-	-	192	-	-	
Mov Cap-2 Maneuver	27	46	-	46	41	-	-	-	-	-	-	-	
Stage 1	31	87	-	56	127	-	-	-	-	-	-	-	
Stage 2	285	125	-	234	86	-	-	-	-	-	-	-	
Approach	EB			WB			NB			SB			
HCM Ctrl Dly, s/v	179.5			97.94			0.5			0.14			
HCM LOS	F			F									

Minor Lane/Major Mvmt	NBL	NBT	NBR B	EBLn1	EBLn2V	VBLn1	SBL	SBT	SBR	
Capacity (veh/h)	115	-	-	29	196	61	192	-	-	
HCM Lane V/C Ratio	0.171	-	-	0.826	0.102	0.392	0.063	-	-	
HCM Ctrl Dly (s/v)	42.6	-	-\$	307.9	25.5	97.9	25.1	-	-	
HCM Lane LOS	E	-	-	F	D	F	D	-	-	
HCM 95th %tile Q(veh)	0.6	-	-	2.7	0.3	1.5	0.2	-	-	

Notes ~: Volume exceeds capacity \$: Delay exceeds 300s

+: Computation Not Defined

\*: All major volume in platoon

Int Delay, s/veh	0.1						
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	1	1	<b>*††</b>		٦	***	
Traffic Vol, veh/h	2	2	1604	5	2	2018	
Future Vol, veh/h	2	2	1604	5	2	2018	
Conflicting Peds, #/hr	0	0	0	0	0	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	-	None	-	None	-	None	
Storage Length	0	0	-	-	100	-	
Veh in Median Storage	,# 1	-	0	-	-	0	
Grade, %	0	-	0	-	-	0	
Peak Hour Factor	50	50	97	63	50	96	
Heavy Vehicles, %	0	50	1	20	0	1	
Mvmt Flow	4	4	1654	8	4	2102	

Major/Minor	Minor1	М	ajor1	Ν	lajor2		
Conflicting Flow All	2506	831	0	0	1662	0	
Stage 1	1658	-	-	-	-	-	
Stage 2	849	-	-	-	-	-	
Critical Hdwy	5.7	8.1	-	-	5.3	-	
Critical Hdwy Stg 1	6.6	-	-	-	-	-	
Critical Hdwy Stg 2	6	-	-	-	-	-	
Follow-up Hdwy	3.8	4.4	-	-	3.1	-	
Pot Cap-1 Maneuver	51	201	-	-	189	-	
Stage 1	96	-	-	-	-	-	
Stage 2	349	-	-	-	-	-	
Platoon blocked, %			-	-		-	
Mov Cap-1 Maneuver	- 50	201	-	-	189	-	
Mov Cap-2 Maneuver	82	-	-	-	-	-	
Stage 1	96	-	-	-	-	-	
Stage 2	341	-	-	-	-	-	

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	37.23	0	0.05
HCM LOS	E		

Minor Lane/Major Mvmt	NBT	NBRWBLn1W	/BLn2	SBL	SBT	
Capacity (veh/h)	-	- 82	201	189	-	
HCM Lane V/C Ratio	-	- 0.049	0.02	0.021	-	
HCM Ctrl Dly (s/v)	-	- 51.2	23.3	24.4	-	
HCM Lane LOS	-	- F	С	С	-	
HCM 95th %tile Q(veh)	-	- 0.2	0.1	0.1	-	

Appendix D Capacity Analysis – 2027 Background Conditions 1.4

## Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ŧ	1		\$		٦	<b>***</b>		۲.	<b>**</b>	
Traffic Vol, veh/h	6	1	11	6	0	10	6	1909	3	5	1059	5
Future Vol, veh/h	6	1	11	6	0	10	6	1909	3	5	1059	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	0	-	-	-	100	-	-	100	-	-
Veh in Median Storage	, # -	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	38	25	83	50	92	75	75	94	75	42	86	42
Heavy Vehicles, %	0	0	0	0	0	0	0	2	0	0	1	0
Mvmt Flow	16	4	13	12	0	13	8	2031	4	12	1231	12

Major/Minor	Minor2		1	Minor1		ſ	Major1		Ν	/lajor2			
Conflicting Flow All	2089	3312	622	2567	3316	1017	1243	0	0	2035	0	0	
Stage 1	1261	1261	-	2049	2049	-	-	-	-	-	-	-	
Stage 2	828	2051	-	518	1267	-	-	-	-	-	-	-	
Critical Hdwy	6.4	6.5	7.1	6.4	6.5	7.1	5.3	-	-	5.3	-	-	
Critical Hdwy Stg 1	7.3	5.5	-	7.3	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.7	5.5	-	6.7	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.8	4	3.9	3.8	4	3.9	3.1	-	-	3.1	-	-	
Pot Cap-1 Maneuver	57	9	372	29	9	205	303	-	-	123	-	-	
Stage 1	133	244	-	36	100	-	-	-	-	-	-	-	
Stage 2	304	100	-	469	242	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	· 47	8	372	23	8	205	303	-	-	123	-	-	
Mov Cap-2 Maneuver	93	59	-	32	64	-	-	-	-	-	-	-	
Stage 1	120	220	-	35	97	-	-	-	-	-	-	-	
Stage 2	277	97	-	401	219	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Ctrl Dly, s/v	42.79	110.04	0.07	0.35	
HCM LOS	E	F			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2V	VBLn1	SBL	SBT	SBR
Capacity (veh/h)	303	-	-	83	372	57	123	-	-
HCM Lane V/C Ratio	0.026	-	-	0.238	0.036	0.441	0.097	-	-
HCM Ctrl Dly (s/v)	17.2	-	-	61.4	15	110	37.4	-	-
HCM Lane LOS	С	-	-	F	С	F	Е	-	-
HCM 95th %tile Q(veh)	0.1	-	-	0.8	0.1	1.7	0.3	-	-

0.9					
WBL	WBR	NBT	NBR	SBL	SBT
<u>۲</u>	1	<b>**i</b>		<u> </u>	***
7	11	1908	4	4	1083
7	11	1908	4	4	1083
0	0	0	0	0	0
Stop	Stop	Free	Free	Free	Free
-	None	-	None	-	None
0	0	-	-	100	-
,# 1	-	0	-	-	0
0	-	0	-	-	0
35	63	94	100	38	84
0	0	2	0	0	1
20	17	2030	4	11	1289
	0.9 WBL 7 7 0 Stop - 0 , # 1 0 35 0 20	0.9 WBL WBR 7 11 7 11 0 0 Stop Stop - None 0 0 ,# 1 - 0 - 35 63 0 0 20 17	0.9 WBL WBR NBT Y 11 1908 7 11 1908 7 11 1908 0 0 0 Stop Stop Free None - None - 0 0 - , # 1 - 0 0 35 63 94 0 22 20 17 2030	0.9         NBT         NBR           WBL         WBR         NBT         NBR           1         1908         4           7         11         1908         4           7         111         1908         4           0         0         0         0           Stop         Stop         Free         Free           None         -         None         -           0         0         -         -           1         -         0         -           0         0         -         -           35         63         94         100           0         0         2         0           20         17         2030         4	0.9       NBT       NBR       SBL         WBL       WBR       NBT       NBR       SBL         T       T       T       T       T       T         T       11       1908       4       4         O       0       0       0       0         Stop       Stop       Free       Free       Free         None       -       None       -       100         M       -       0       -       100       -         M       -       0       -       -       -         M       -       0       -       -       -         M       -       0       -       -       -         M       -       0       -       -       -         M       -       0       -       -       -         M       -       0       -       -       -         M       -       0       -

Major/Minor	Minor1	Ν	1ajor1	Ν	/lajor2		
Conflicting Flow All	2569	1017	0	0	2034	0	
Stage 1	2032	-	-	-	-	-	
Stage 2	537	-	-	-	-	-	
Critical Hdwy	5.7	7.1	-	-	5.3	-	
Critical Hdwy Stg 1	6.6	-	-	-	-	-	
Critical Hdwy Stg 2	6	-	-	-	-	-	
Follow-up Hdwy	3.8	3.9	-	-	3.1	-	
Pot Cap-1 Maneuver	47	205	-	-	123	-	
Stage 1	55	-	-	-	-	-	
Stage 2	507	-	-	-	-	-	
Platoon blocked, %			-	-		-	
Mov Cap-1 Maneuver	r 43	205	-	-	123	-	
Mov Cap-2 Maneuver	r 50	-	-	-	-	-	
Stage 1	55	-	-	-	-	-	
Stage 2	464	-	-	-	-	-	

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	74.01	0	0.3
HCM LOS	F		

Minor Lane/Major Mvmt	NBT	NBRWBLn1	VBLn2	SBL	SBT	
Capacity (veh/h)	-	- 50	205	123	-	
HCM Lane V/C Ratio	-	- 0.397	0.085	0.085	-	
HCM Ctrl Dly (s/v)	-	- 117.5	24.2	36.9	-	
HCM Lane LOS	-	- F	С	Е	-	
HCM 95th %tile Q(veh)	-	- 1.4	0.3	0.3	-	

4.5

## Intersection

Int Delay, s/veh

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ŧ	1		4		٦	<b>*††</b>		ኘ	<b>***</b>	
Traffic Vol, veh/h	11	2	12	9	2	5	12	1706	10	4	2133	9
Future Vol, veh/h	11	2	12	9	2	5	12	1706	10	4	2133	9
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	0	-	-	-	100	-	-	100	-	-
Veh in Median Storage,	# -	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	50	50	55	67	50	63	56	98	45	33	96	67
Heavy Vehicles, %	0	0	0	0	0	20	0	1	11	0	1	0
Mvmt Flow	22	4	22	13	4	8	21	1741	22	12	2222	13

Major/Minor	Minor2		I	Minor1		I	Major1		Ν	/lajor2			
Conflicting Flow All	2994	4059	1118	2710	4054	882	2235	0	0	1763	0	0	
Stage 1	2253	2253	-	1795	1795	-	-	-	-	-	-	-	
Stage 2	741	1806	-	915	2260	-	-	-	-	-	-	-	
Critical Hdwy	6.4	6.5	7.1	6.4	6.5	7.5	5.3	-	-	5.3	-	-	
Critical Hdwy Stg 1	7.3	5.5	-	7.3	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.7	5.5	-	6.7	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.8	4	3.9	3.8	4	4.1	3.1	-	-	3.1	-	-	
Pot Cap-1 Maneuver	~ 15	~ 3	176	23	~ 3	222	97	-	-	168	-	-	
Stage 1	26	79	-	55	134	-	-	-	-	-	-	-	
Stage 2	344	132	-	269	78	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	· ~ 10	~ 2	176	14	~ 2	222	97	-	-	168	-	-	
Mov Cap-2 Maneuver	~ 21	38	-	36	32	-	-	-	-	-	-	-	
Stage 1	24	73	-	43	104	-	-	-	-	-	-	-	
Stage 2	249	103	-	207	72	-	-	-	-	-	-	-	
Approach	EB			WB			NB			SB			
HCM Ctrl Dly, s/v	277.96		1	48.98			0.63			0.15			
HCM LOS	F			F									

Capacity (veh/h)         97         -         -         23         176         47         168         -         -           HCM Lane V/C Ratio         0.22         -         -         1.151         0.124         0.539         0.072         -         -           HCM Ctrl Dly (s/v)         52.1         -         -         \$ 487.4         28.4         149         28         -         -
HCM Lane V/C Ratio         0.22         -         -         1.151         0.124         0.539         0.072         -         -           HCM Ctrl Dly (s/v)         52.1         -         -\$ 487.4         28.4         149         28         -         -
HCM Ctrl Dly (s/v) 52.1\$ 487.4 28.4 149 28
HCM Lane LOS F F D F D
HCM 95th %tile Q(veh) 0.8 3.3 0.4 2 0.2

Notes ~: Volume exceeds capacity \$: Delay exceeds 300s

+: Computation Not Defined

\*: All major volume in platoon

Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	<u>الا</u>	1	<b>**i</b>		<u>آ</u>	***
Traffic Vol, veh/h	2	2	1712	5	2	2154
Future Vol, veh/h	2	2	1712	5	2	2154
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	100	-
Veh in Median Storage	e, # 1	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	50	50	97	63	50	96
Heavy Vehicles, %	0	50	1	20	0	1
Mvmt Flow	4	4	1765	8	4	2244

Major/Minor	Minor1	М	ajor1	Ν	lajor2		
Conflicting Flow All	2674	886	0	0	1773	0	
Stage 1	1769	-	-	-	-	-	
Stage 2	906	-	-	-	-	-	
Critical Hdwy	5.7	8.1	-	-	5.3	-	
Critical Hdwy Stg 1	6.6	-	-	-	-	-	
Critical Hdwy Stg 2	6	-	-	-	-	-	
Follow-up Hdwy	3.8	4.4	-	-	3.1	-	
Pot Cap-1 Maneuver	41	182	-	-	167	-	
Stage 1	82	-	-	-	-	-	
Stage 2	325	-	-	-	-	-	
Platoon blocked, %			-	-		-	
Mov Cap-1 Maneuve	r 40	182	-	-	167	-	
Mov Cap-2 Maneuve	r 70	-	-	-	-	-	
Stage 1	82	-	-	-	-	-	
Stage 2	317	-	-	-	-	-	

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	42.5	0	0.05
HCMLOS	F		

Minor Lane/Major Mvmt	NBT	NBR	VBLn1V	VBLn2	SBL	SBT
Capacity (veh/h)	-	-	70	182	167	-
HCM Lane V/C Ratio	-	-	0.057	0.022	0.024	-
HCM Ctrl Dly (s/v)	-	-	59.8	25.2	27.1	-
HCM Lane LOS	-	-	F	D	D	-
HCM 95th %tile Q(veh)	-	-	0.2	0.1	0.1	-

Appendix E Capacity Analysis – 2027 Projected Conditions 1.5

Intersection

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ŧ	1		4		٦	<b>*††</b>		٦	<b>**</b>	
Traffic Vol, veh/h	6	1	11	6	0	10	6	1926	3	5	1065	5
Future Vol, veh/h	6	1	11	6	0	10	6	1926	3	5	1065	5
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	0	-	-	-	100	-	-	100	-	-
Veh in Median Storage	, # -	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	38	25	83	50	92	75	75	94	75	42	86	42
Heavy Vehicles, %	0	0	0	0	0	0	0	2	0	0	1	0
Mvmt Flow	16	4	13	12	0	13	8	2049	4	12	1238	12

Major/Minor	Minor2		1	Minor1		ľ	Major1		Ν	/lajor2			
Conflicting Flow All	2104	3337	625	2588	3341	1026	1250	0	0	2053	0	0	
Stage 1	1268	1268	-	2067	2067	-	-	-	-	-	-	-	
Stage 2	836	2069	-	521	1274	-	-	-	-	-	-	-	
Critical Hdwy	6.4	6.5	7.1	6.4	6.5	7.1	5.3	-	-	5.3	-	-	
Critical Hdwy Stg 1	7.3	5.5	-	7.3	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.7	5.5	-	6.7	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.8	4	3.9	3.8	4	3.9	3.1	-	-	3.1	-	-	
Pot Cap-1 Maneuver	56	8	370	28	8	202	301	-	-	121	-	-	
Stage 1	131	242	-	35	98	-	-	-	-	-	-	-	
Stage 2	301	97	-	467	240	-	-	-	-	-	-	-	
Platoon blocked, %								-	-		-	-	
Mov Cap-1 Maneuver	46	7	370	22	7	202	301	-	-	121	-	-	
Mov Cap-2 Maneuver	91	57	-	31	62	-	-	-	-	-	-	-	
Stage 1	118	218	-	34	95	-	-	-	-	-	-	-	
Stage 2	274	95	-	398	216	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Ctrl Dly, s/v	43.56	114.69	0.07	0.36	
HCM LOS	E	F			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2V	VBLn1	SBL	SBT	SBR
Capacity (veh/h)	301	-	-	82	370	56	121	-	-
HCM Lane V/C Ratio	0.027	-	-	0.242	0.036	0.454	0.099	-	-
HCM Ctrl Dly (s/v)	17.3	-	-	62.6	15.1	114.7	38.1	-	-
HCM Lane LOS	С	-	-	F	С	F	Е	-	-
HCM 95th %tile Q(veh)	0.1	-	-	0.9	0.1	1.7	0.3	-	-

0.8					
WBL	WBR	NBT	NBR	SBL	SBT
<u>الا</u>	1	<b>**i</b>		<u>آ</u>	***
14	17	1918	5	6	1076
14	17	1918	5	6	1076
0	0	0	0	0	0
Stop	Stop	Free	Free	Free	Free
-	None	-	None	-	None
0	0	-	-	100	-
,# 1	-	0	-	-	0
0	-	0	-	-	0
92	92	92	92	92	92
2	2	2	2	2	2
15	18	2085	5	7	1170
	0.8 WBL 14 14 0 Stop - 0 *, # 1 0 92 2 2 15	0.8 WBL WBR 14 17 14 17 14 17 0 0 Stop Stop Stop 0 Stop 0 4 1 0 - 0 92 92 2 2 15 18	0.8 WBL WBR NBT 14 17 1918 14 17 1918 14 17 1918 0 0 0 Stop Stop Free None - 0 0 - 4, # 1 - 0 0 0 - 92 92 92 92 2 2 2 15 18 2085	0.8         NBT         NBR           WBL         WBR         NBT         NBR           14         17         1918         5           14         17         1918         5           14         17         1918         5           0         0         0         0           Stop         Stop         Free         Free           None         -         None         -           0         0         -         -           4, #1         -         0         -           90         0         -         -           92         92         92         92           92         2         2         2           15         18         2085         5	0.8       NBR       NBR       SBL         WBL       WBR       NBT       NBR       SBL         14       17       1918       5       6         14       17       1918       5       6         14       17       1918       5       6         0       0       0       0       0         Stop       Stop       Free       Free       Free         None       -       None       -       100         4       1       0       -       100       -         5       0       -       0       -       -         0       0       -       0       -       -         92       92       92       92       92       92         22       2       2       2       2       2         15       18       2085       5       7

Major/Minor	Minor1	Ν	/lajor1	Ν	1ajor2		
Conflicting Flow All	2568	1045	0	0	2090	0	
Stage 1	2088	-	-	-	-	-	
Stage 2	481	-	-	-	-	-	
Critical Hdwy	5.74	7.14	-	-	5.34	-	
Critical Hdwy Stg 1	6.64	-	-	-	-	-	
Critical Hdwy Stg 2	6.04	-	-	-	-	-	
Follow-up Hdwy	3.82	3.92	-	-	3.12	-	
Pot Cap-1 Maneuver	46	194	-	-	112	-	
Stage 1	50	-	-	-	-	-	
Stage 2	537	-	-	-	-	-	
Platoon blocked, %	1	1	-	-	1	-	
Mov Cap-1 Maneuver	• 43	194	-	-	112	-	
Mov Cap-2 Maneuver	- 45	-	-	-	-	-	
Stage 1	50	-	-	-	-	-	
Stage 2	506	-	-	-	-	-	

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	68.19	0	0.22
HCMLOS	F		

Minor Lane/Major Mvmt	NBT	NBR	VBLn1V	VBLn2	SBL	SBT	
Capacity (veh/h)	-	-	45	194	112	-	
HCM Lane V/C Ratio	-	-	0.335	0.095	0.058	-	
HCM Ctrl Dly (s/v)	-	-	120	25.6	39	-	
HCM Lane LOS	-	-	F	D	E	-	
HCM 95th %tile Q(veh)	-	-	1.2	0.3	0.2	-	

Int Delay, s/veh	0.9								
Movement	WBL	WBR	NBT	NBR	SBL	SBT			
Lane Configurations	٦	1	<b>**i</b>		٦	***			
Traffic Vol, veh/h	7	11	1913	4	4	1097			
Future Vol, veh/h	7	11	1913	4	4	1097			
Conflicting Peds, #/hr	0	0	0	0	0	0			
Sign Control	Stop	Stop	Free	Free	Free	Free			
RT Channelized	-	None	-	None	-	None			
Storage Length	0	0	-	-	100	-			
Veh in Median Storage	, # 1	-	0	-	-	0			
Grade, %	0	-	0	-	-	0			
Peak Hour Factor	35	63	94	100	38	84			
Heavy Vehicles, %	0	0	2	0	0	1			
Mvmt Flow	20	17	2035	4	11	1306			

Major/Minor	Minor1	Ν	1ajor1	Ν	lajor2		
Conflicting Flow All	2581	1020	0	0	2039	0	
Stage 1	2037	-	-	-	-	-	
Stage 2	543	-	-	-	-	-	
Critical Hdwy	5.7	7.1	-	-	5.3	-	
Critical Hdwy Stg 1	6.6	-	-	-	-	-	
Critical Hdwy Stg 2	6	-	-	-	-	-	
Follow-up Hdwy	3.8	3.9	-	-	3.1	-	
Pot Cap-1 Maneuver	46	204	-	-	122	-	
Stage 1	55	-	-	-	-	-	
Stage 2	503	-	-	-	-	-	
Platoon blocked, %	1	1	-	-		-	
Mov Cap-1 Maneuver	r 42	204	-	-	122	-	
Mov Cap-2 Maneuver	r 50	-	-	-	-	-	
Stage 1	55	-	-	-	-	-	
Stage 2	460	-	-	-	-	-	

Approach	WB	NB	SB	
HCM Ctrl Dly, s/v	74.78	0	0.3	
HCM LOS	F			

Minor Lane/Major Mvmt	NBT	NBRWBLn1	VBLn2	SBL	SBT	
Capacity (veh/h)	-	- 50	204	122	-	
HCM Lane V/C Ratio	-	- 0.401	0.086	0.086	-	
HCM Ctrl Dly (s/v)	-	- 118.9	24.3	37.1	-	
HCM Lane LOS	-	- F	С	E	-	
HCM 95th %tile Q(veh)	-	- 1.4	0.3	0.3	-	

4.7

## Intersection

Int Delay, s/veh

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ŧ	1		\$		۲.	<b>***</b>		۲.	<b>**</b>	
Traffic Vol, veh/h	11	2	12	9	2	5	12	1716	10	4	2151	9
Future Vol, veh/h	11	2	12	9	2	5	12	1716	10	4	2151	9
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	0	-	-	-	100	-	-	100	-	-
Veh in Median Storage,	# -	1	-	-	1	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	50	50	55	67	50	63	56	98	45	33	96	67
Heavy Vehicles, %	0	0	0	0	0	20	0	1	11	0	1	0
Mvmt Flow	22	4	22	13	4	8	21	1751	22	12	2241	13

Major/Minor	Minor2		ľ	Minor1		ľ	Major1		Ν	/lajor2			
Conflicting Flow All	3017	4088	1127	2727	4083	887	2254	0	0	1773	0	0	
Stage 1	2272	2272	-	1805	1805	-	-	-	-	-	-	-	
Stage 2	745	1816	-	922	2278	-	-	-	-	-	-	-	
Critical Hdwy	6.4	6.5	7.1	6.4	6.5	7.5	5.3	-	-	5.3	-	-	
Critical Hdwy Stg 1	7.3	5.5	-	7.3	5.5	-	-	-	-	-	-	-	
Critical Hdwy Stg 2	6.7	5.5	-	6.7	5.5	-	-	-	-	-	-	-	
Follow-up Hdwy	3.8	4	3.9	3.8	4	4.1	3.1	-	-	3.1	-	-	
Pot Cap-1 Maneuver	~ 15	~ 3	173	23	~ 3	220	95	-	-	166	-	-	
Stage 1	25	77	-	55	132	-	-	-	-	-	-	-	
Stage 2	342	131	-	266	76	-	-	-	-	-	-	-	
Platoon blocked, %	1							-	-		-	-	
Mov Cap-1 Maneuver	~ 9	~ 2	173	~ 13	~ 2	220	95	-	-	166	-	-	
Mov Cap-2 Maneuver	~ 20	37	-	35	31	-	-	-	-	-	-	-	
Stage 1	23	71	-	42	103	-	-	-	-	-	-	-	
Stage 2	245	101	-	204	71	-	-	-	-	-	-	-	

Approach	EB	WB	NB	SB	
HCM Ctrl Dly, s/v	290.99	154.66	0.64	0.15	
HCM LOS	F	F			

Minor Lane/Major Mvmt	NBL	NBT	NBR	EBLn1	EBLn2\	WBLn1	SBL	SBT	SBR
Capacity (veh/h)	95	-	-	22	173	46	166	-	-
HCM Lane V/C Ratio	0.225	-	-	1.188	0.126	0.552	0.073	-	-
HCM Ctrl Dly (s/v)	53.4	-	-	\$ 511	28.8	154.7	28.3	-	-
HCM Lane LOS	F	-	-	F	D	F	D	-	-
HCM 95th %tile Q(veh)	0.8	-	-	3.4	0.4	2.1	0.2	-	-

Notes ~: Volume exceeds capacity \$: Delay exceeds 300s

+: Computation Not Defined

\*: All major volume in platoon

Int Delay, s/veh	0.4					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	<u>الا</u>	1	<b>**i</b>		<u> </u>	***
Traffic Vol, veh/h	9	10	1714	15	18	2153
Future Vol, veh/h	9	10	1714	15	18	2153
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	0	-	-	100	-
Veh in Median Storage	, # 1	-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	10	11	1863	16	20	2340

Major/Minor	Minor1	Ν	1ajor1	Ν	/lajor2			
Conflicting Flow All	2846	940	0	0	1879	0		
Stage 1	1871	-	-	-	-	-		
Stage 2	975	-	-	-	-	-		
Critical Hdwy	5.74	7.14	-	-	5.34	-		
Critical Hdwy Stg 1	6.64	-	-	-	-	-		
Critical Hdwy Stg 2	6.04	-	-	-	-	-		
Follow-up Hdwy	3.82	3.92	-	-	3.12	-		
Pot Cap-1 Maneuver	32	228	-	-	144	-		
Stage 1	69	-	-	-	-	-		
Stage 2	295	-	-	-	-	-		
Platoon blocked, %			-	-		-		
Mov Cap-1 Maneuver	r 28	228	-	-	144	-		
Mov Cap-2 Maneuver	r 57	-	-	-	-	-		
Stage 1	69	-	-	-	-	-		
Stage 2	255	-	-	-	-	-		

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	49.65	0	0.28
HCM LOS	E		

Minor Lane/Major Mvmt	NBT	NBRW	BLn1V	VBLn2	SBL	SBT	
Capacity (veh/h)	-	-	57	228	144	-	
HCM Lane V/C Ratio	-	- (	).172	0.048	0.136	-	
HCM Ctrl Dly (s/v)	-	-	80.8	21.6	33.9	-	
HCM Lane LOS	-	-	F	С	D	-	
HCM 95th %tile Q(veh)	-	-	0.6	0.1	0.5	-	

Int Delay, s/veh	0.1							
Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	۲.	1	<b>**</b>		٦	***		
Traffic Vol, veh/h	2	2	1727	5	2	2163		
Future Vol, veh/h	2	2	1727	5	2	2163		
Conflicting Peds, #/hr	0	0	0	0	0	0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	-	None	-	None	-	None		
Storage Length	0	0	-	-	100	-		
Veh in Median Storage	e, # 1	-	0	-	-	0		
Grade, %	0	-	0	-	-	0		
Peak Hour Factor	50	50	97	63	50	96		
Heavy Vehicles, %	0	50	1	20	0	1		
Mvmt Flow	4	4	1780	8	4	2253		

Major/Minor	Minor1	М	ajor1	Ν	lajor2	
Conflicting Flow All	2694	894	0	0	1788	0
Stage 1	1784	-	-	-	-	-
Stage 2	909	-	-	-	-	-
Critical Hdwy	5.7	8.1	-	-	5.3	-
Critical Hdwy Stg 1	6.6	-	-	-	-	-
Critical Hdwy Stg 2	6	-	-	-	-	-
Follow-up Hdwy	3.8	4.4	-	-	3.1	-
Pot Cap-1 Maneuver	40	180	-	-	164	-
Stage 1	80	-	-	-	-	-
Stage 2	324	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuve	r 39	180	-	-	164	-
Mov Cap-2 Maneuve	r 68	-	-	-	-	-
Stage 1	80	-	-	-	-	-
Stage 2	316	-	-	-	-	-

Approach	WB	NB	SB
HCM Ctrl Dly, s/v	43.28	0	0.05
HCMLOS	E		

Minor Lane/Major Mvmt	NBT	NBRWBLn	WBLn2	SBL	SBT	
Capacity (veh/h)	-	- 68	8 180	164	-	
HCM Lane V/C Ratio	-	- 0.059	0.022	0.024	-	
HCM Ctrl Dly (s/v)	-	- 61.1	25.5	27.6	-	
HCM Lane LOS	-	- F	D	D	-	
HCM 95th %tile Q(veh)	-	- 0.2	2 0.1	0.1	-	

Appendix F Trip Generation Reports

		PROJECT DETAILS	
Project Name:	Armstrong Hills	Type of Project:	
Project No:		City:	
Country:		Built-up Area(Sq.ft):	
Analyst Name:	Colby Wright	Clients Name:	
Date:	11/6/2024	ZIP/Postal Code:	
State/Province:		No. of Scenarios: 3	
Analysis Region:			
		SCENARIO SUMMARY	

Scenarios	Name	No. of Land Lises	Phases of	No. of Years to Project	User Group	Estimated New Vehicle Trips			
	Name	No. of Land Oses	Development	Traffic		Entry	Exit	Total	
Scenario - 1	AM Peak Hour	1	1	0		10	30	40	
Scenario - 2	PM Peak Hour	1	1	0		33	19	52	
Scenario - 3	Weekday	1	1	0		267	267	534	

#### Scenario - 1

Scenario Name:	Peak Hour User Group:
Dev. phase:	No. of Years to Project 0 Traffic :
Analyst Note:	
Warning:	

#### VEHICLE TRIPS BEFORE REDUCTION

Land Lica & Data Cource	location	IV Size	Time Deried	Method	Entry	Exit	Total	
	Location		Size	Time Period	Rate/Equation	Split%	Split%	Total
210 - Single-Family Detached Housing	General	Dwolling Units	FO	Weekday, Peak Hour of	Best Fit (LOG)	10	30	40
Data Source: Trip Generation Manual, 11th Ed	Urban/Suburban	Dwening Onits	g Units 50	Adjacent Street Traffic,	Ln(T) =0.91Ln(X) + 0.12	25%	75%	40

#### VEHICLE TO PERSON TRIP CONVERSION

#### BASELINE SITE VEHICLE CHARACTERISTICS:

Land Lico	Baseline Site Vehicle Mode Share		Baseline Site Vehicle Occupancy		Baseline Site Vehicle Directional Split	
	Entry (%)	Exit (%)	Entry	Exit	Entry (%)	Exit (%)
210 - Single-Family Detached Housing	100	100	1	1	25	75

#### ESTIMATED BASELINE SITE PERSON TRIPS:

l and lico	Person Trips by Vehicle		Person Trips by Other Modes		Total Baseline Site Person Trips	
	Entry	Exit	Entry	Exit	Total Baseline S Entry 10	Exit
210 Single Femily Detected Heuring	10	30	0	0	10	30
210 - Single-Failing Detached Housing	40		0		40	

#### NEW VEHICLE TRIPS

Land Line		New Vehicle Trips				
	Entry	Exit	Total			
210 - Single-Family Detached Housing	10	30	40			

#### RESULTS

Site Totals	Entry	Exit	Total
Vehicle Trips Before Reduction	10	30	40
External Vehicle Trips	10	30	40
New Vehicle Trips	10	30	40

#### Scenario - 2

Scenario Name:	PM Peak Hour	User Group:	
Dev. phase:	1	No. of Years to Project Traffic :	
Analyst Note:			
Warning:			

#### VEHICLE TRIPS BEFORE REDUCTION

Land Line & Date Course	I continu	IV Size	6:	Time Period -	Method	Entry	Exit	Total	
Land Use & Data Source	Location		Size		Rate/Equation	Split%	Split%	Iotai	
210 - Single-Family Detached Housing	General	Dwolling Units	FO	Weekday, Peak Hour of	Best Fit (LOG)	33	19	E2	
Data Source: Trip Generation Manual, 11th Ed	Urban/Suburban	Dwening Units	50	Adjacent Street Traffic,	Ln(T) =0.94Ln(X) + 0.27	63%	37%	52	

#### VEHICLE TO PERSON TRIP CONVERSION

#### BASELINE SITE VEHICLE CHARACTERISTICS:

Land Lico	Baseline Site Vehicle Mode Share		Baseline Site Vehicle Occupancy		Baseline Site Vehicle Directional Split	
	Entry (%)	Exit (%)	Entry	Exit	Entry (%)	Exit (%)
210 - Single-Family Detached Housing	100	100	1	1	63	37

#### ESTIMATED BASELINE SITE PERSON TRIPS:

Landlico	Person Trips by Vehicle		Person Trips by Other Modes		Total Baseline Site Person Trips	
	Entry	Exit	Entry	Exit	Total Baseline S Entry 33 5	Exit
210 Single Family Detached Housing	33	19	0	0	33	19
210 - Single-Failing Detached Housing	52		0		52	

#### NEW VEHICLE TRIPS

Land Uro		New Vehicle Trips				
	Entry	Exit	Total			
210 - Single-Family Detached Housing	33	19	52			

#### RESULTS

Site Totals	Entry	Exit	Total
Vehicle Trips Before Reduction	33	19	52
External Vehicle Trips	33	19	52
New Vehicle Trips	33	19	52

# Scenario - 3 User Group: Scenario Name: Weekday User Group: Dev. phase: 1 No. of Years to Project Traffic : Analyst Note: 0

# Warning:

Land Use & Data Source	Location	IV	Size	Time Period	Method	Entry	Exit	- Total
					Rate/Equation	Split%	Split%	
210 - Single-Family Detached Housing	General	Dwelling Units	50	Weekday	Best Fit (LOG)	267	267	534
Data Source: Trip Generation Manual, 11th Ed	Urban/Suburban				Ln(T) =0.92Ln(X) + 2.68	50%	50%	

#### VEHICLE TO PERSON TRIP CONVERSION

#### BASELINE SITE VEHICLE CHARACTERISTICS:

landlico	Baseline Site Vehicle Mode Share		Baseline Site Vehicle Occupancy		Baseline Site Vehicle Directional Split	
	Entry (%)	Exit (%)	Entry	Exit	Entry (%)	Exit (%)
210 - Single-Family Detached Housing	100	100	1	1	50	50

#### ESTIMATED BASELINE SITE PERSON TRIPS:

landlico	Person Trips by Vehicle		Person Trips by Other Modes		Total Baseline Site Person Trips	
	Entry	Exit	Entry	Exit	Total Baseline Si Entry 267 53	Exit
210 Single Family Detected Housing	267	267	0	0	267	267
210 - Single-Family Detached Housing	534		0		534	

#### NEW VEHICLE TRIPS

Land Like	New Vehicle Trips			
	Entry	Exit	Total	
210 - Single-Family Detached Housing	267	267	534	

#### RESULTS

Site Totals	Entry	Exit	Total
Vehicle Trips Before Reduction	267	267	534
External Vehicle Trips	267	267	534
New Vehicle Trips	267	267	534

Appendix G FM 1938 Raised Median Project Fact Sheet and Schematic

# FM 1938 (Davis Boulevard) Raised Median Project



## **Project Overview**

The Texas Department of Transportation (TxDOT) is proposing improvements to FM 1938 (Davis Boulevard) from FM 1709 (Southlake Boulevard) to Emerald Hills Way in Tarrant County, Texas. The proposed project would convert the two-way left turn lane into a 14-foot-wide raised median.

At intersections with median breaks, the median would be reduced to three-foot-wide, and an 11-foot-wide left turn lane would be provided. Lane configuration would not change and would still provide three 11.5-foot-wide travel lanes in each direction. The purpose of the proposed project is to improve safety, connectivity, and mobility by accommodating traffic growth and controlling turning movements to reduce crashes.

# **Project Goals**

## Need

- High crash rates due to uncontrolled turning movements
- Increasing traffic volumes
- Increased congestion

## **How to Submit Comments**

You may submit comments in any language in the following ways:

- Place the comment card in the comment box at the in-person meeting.
- Send your comment via U.S. Mail postmarked by Monday, December 2, 2024.

TxDOT For Worth District Office Attn: Mohammad Faysal, P.E. 2501 SW Loop 820 Fort Worth, Texas 76133

 Send your comment via email to: md.faysal@txdot.gov

All comments must be received or postmarked by **Monday, December 2, 2024,** to be a part of the official public meeting record.

## **Anticipated Project Timeline\***





\*This timeline is approximate and subject to change.

The environmental review, consultation, and other actions required by applicable Federal environmental laws for this project are being, or have been, carried out by TxDOT pursuant to 23 U.S.C. 327 and a Memorandum of Understanding dated December 9, 2019, and executed by FHWA and TxDOT.











Appendix H Driveway Spacing Diagram





Appendix I Stopping Sight Distance Photos



Northbound Davis Blvd (FM 1938) (425' south of Street A)

Southbound Davis Blvd (FM 1938) (425' north of Street A)



Appendix J Intersection Sight Distance Triangle Diagram and Photos



## LEGEND:

□ PASSENGER CAR

SIGHT TRIANGLE

**PROPOSED DRIVEWAY** 

DAVIS BLVD (FM 1938) AT STREET A SIGHT DISTANCE TRIANGLES




Westbound Right onto Davis Blvd (FM 1938) from Street A

Westbound Left onto Davis Blvd (FM 1938) from Street A

